

# Torsional Loading of Subsea Shallow Foundations

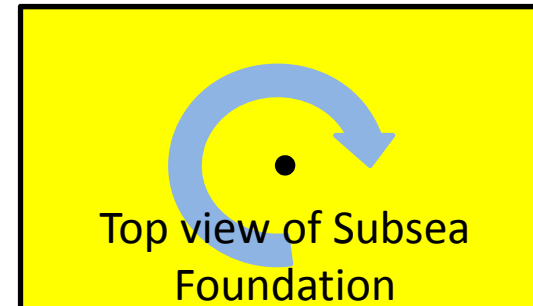
Final year research project  
Ben Holland



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# What is torsion loading?

- Twisting of the foundation about vertical axis



- Torsional loading has been shown to significantly reduce foundation capacity

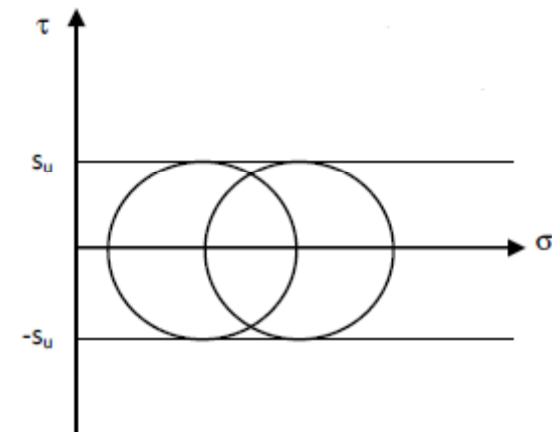
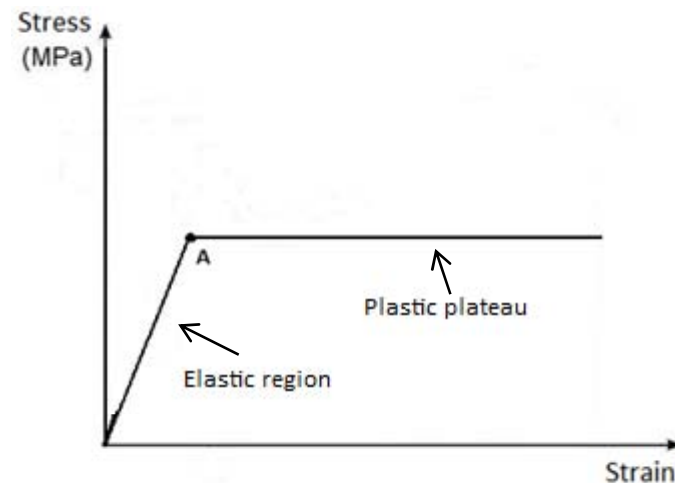


# Project Objectives

1. Define the effect of torsional loading on lateral capacity of shallow foundations.
2. Develop a suitable design technique for combined VHT loading
3. Note the effect of foundation embedment, aspect ratio and soil strength profile

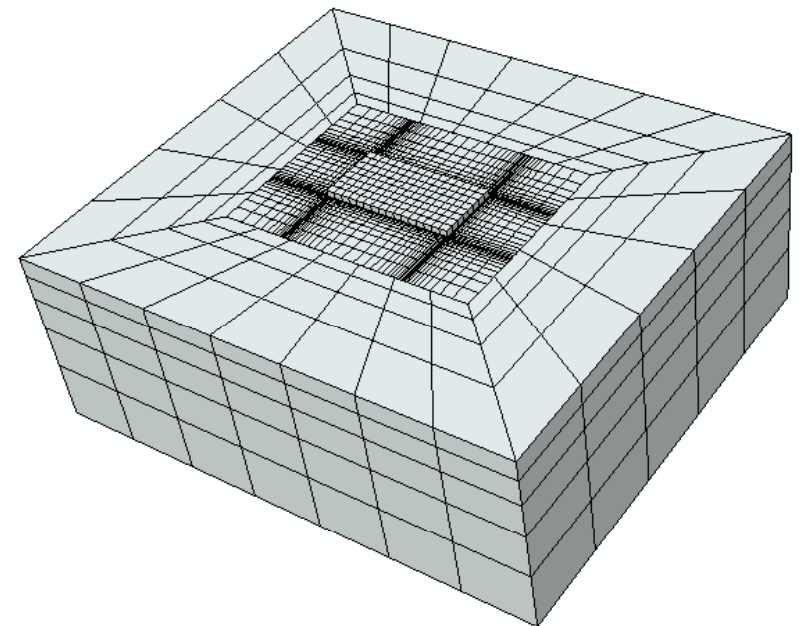
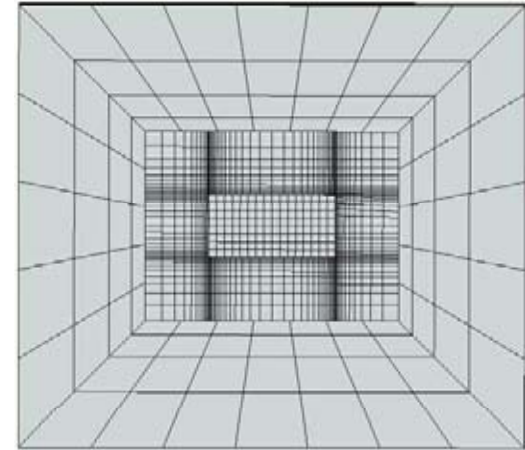
# Study technique

- Analysis conducted using Abaqus FEA software
- Rigid foundation – constant reference point
- Linear elastic perfectly plastic soil
  - Undrained, conforming to Tresca Criterion



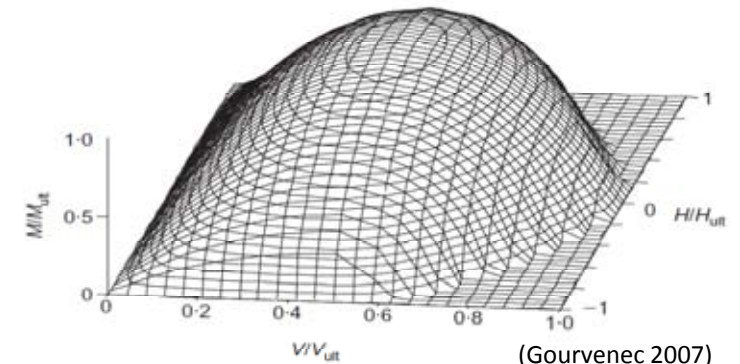
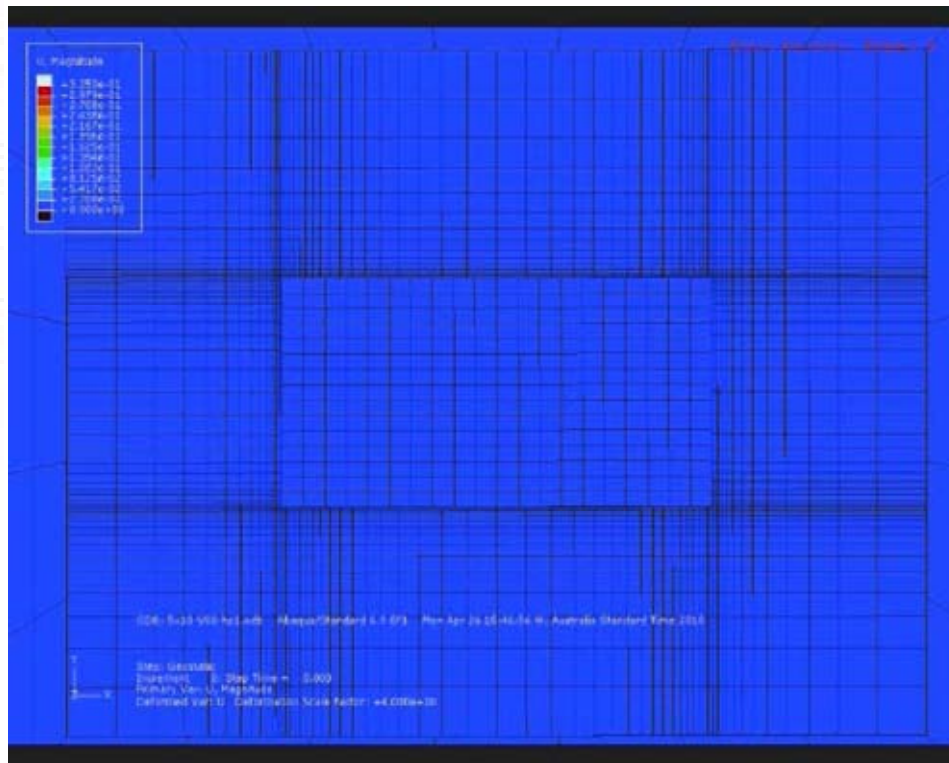
# Models

- 3 aspect ratios
  - $B/L = 0.5, 1, 2$
- 2 embedment ratios
  - Surface and embedded
- 2 soils strength profiles
  - Uniform and highly heterogeneous

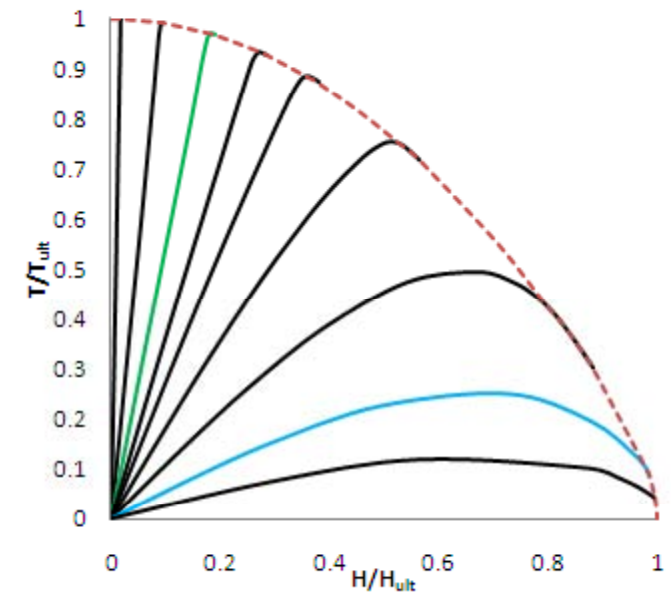


# Failure envelopes

- Failure envelopes are the best way to depict the interaction
  - Normalised loads

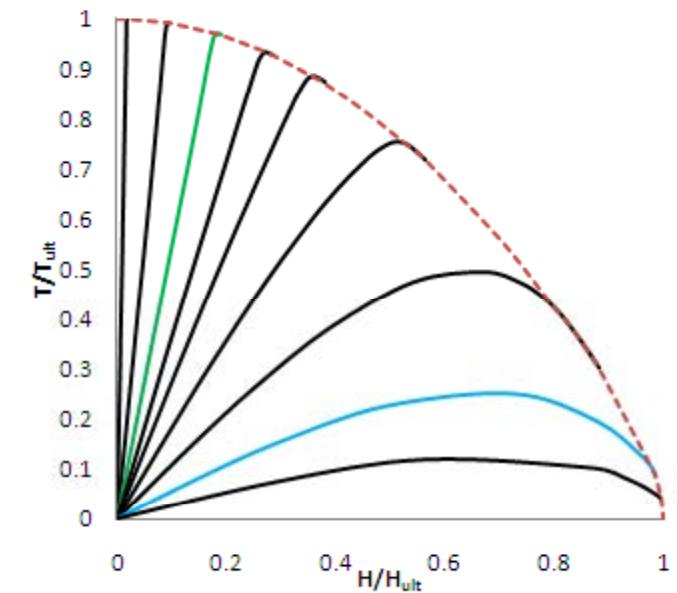
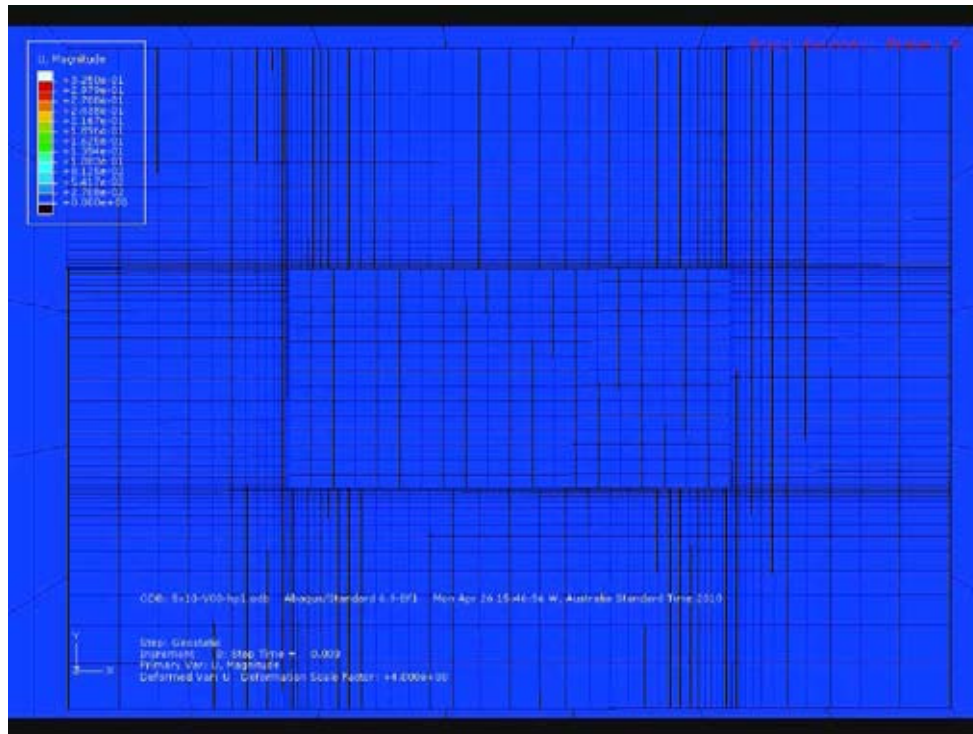


(Gourvenec 2007)



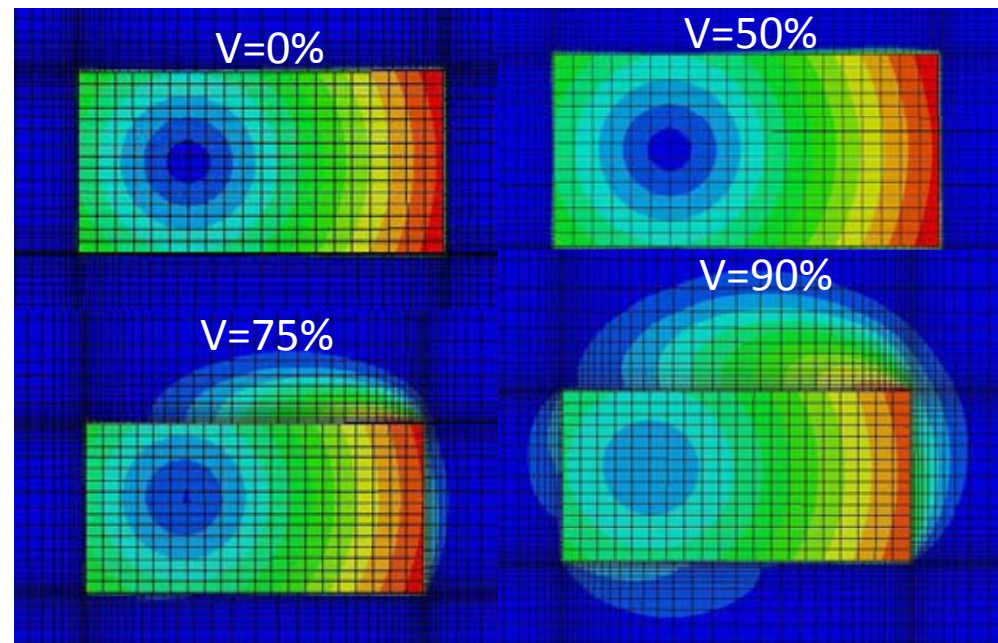
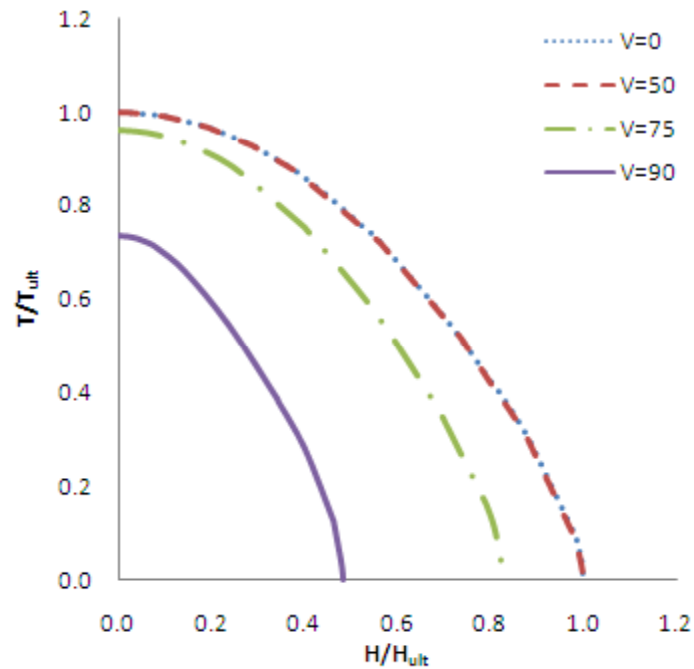
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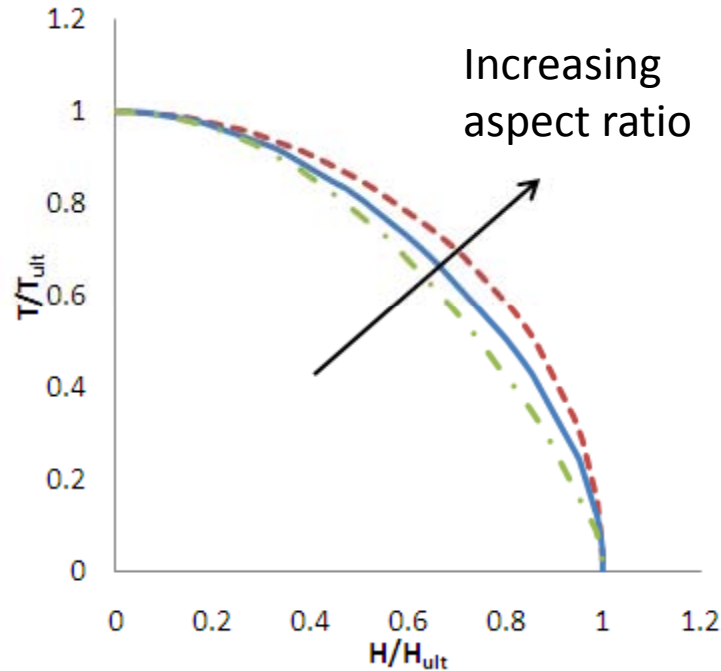
# Effect of Vertical load

- Vertical load reduces the horizontal and torsional capacity

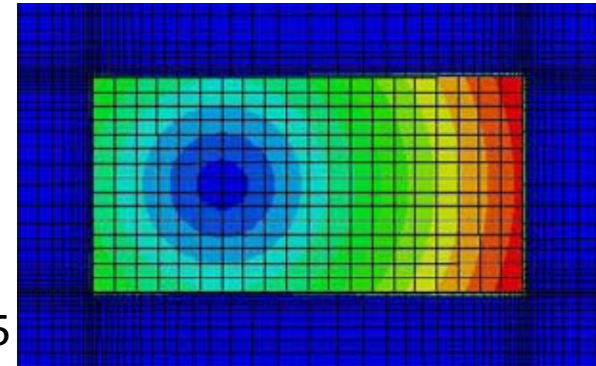


# Foundation aspect ratio

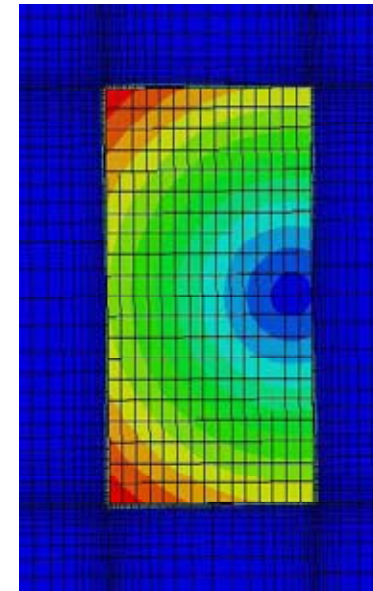
- Increased normalised HT capacity with increasing aspect ratio



$B/L = 0.5$

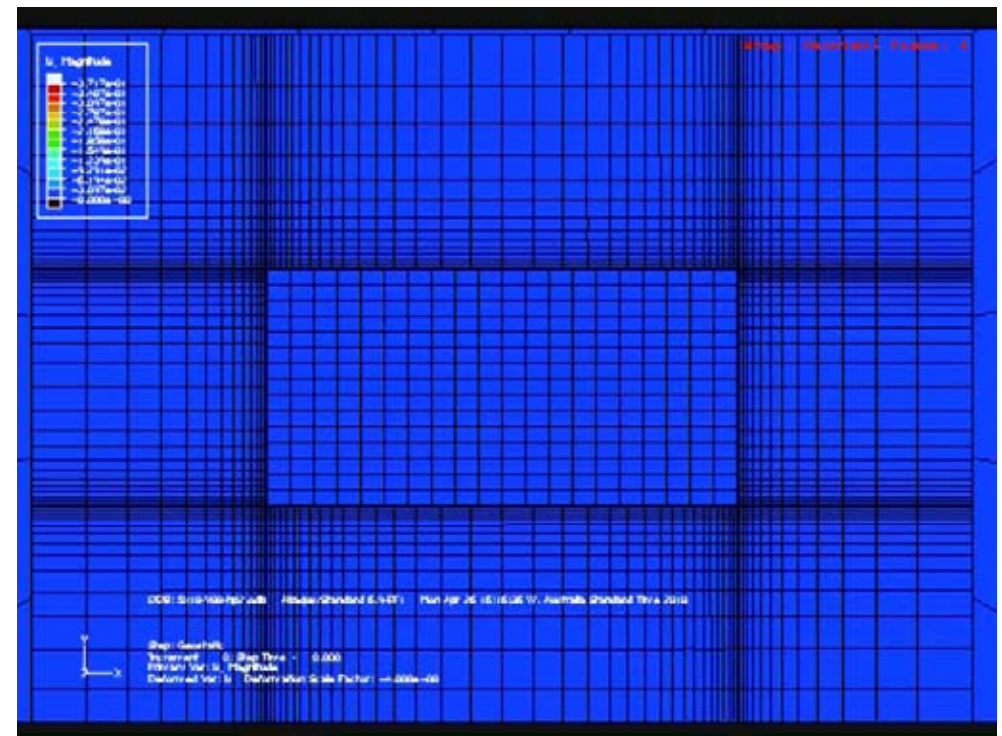
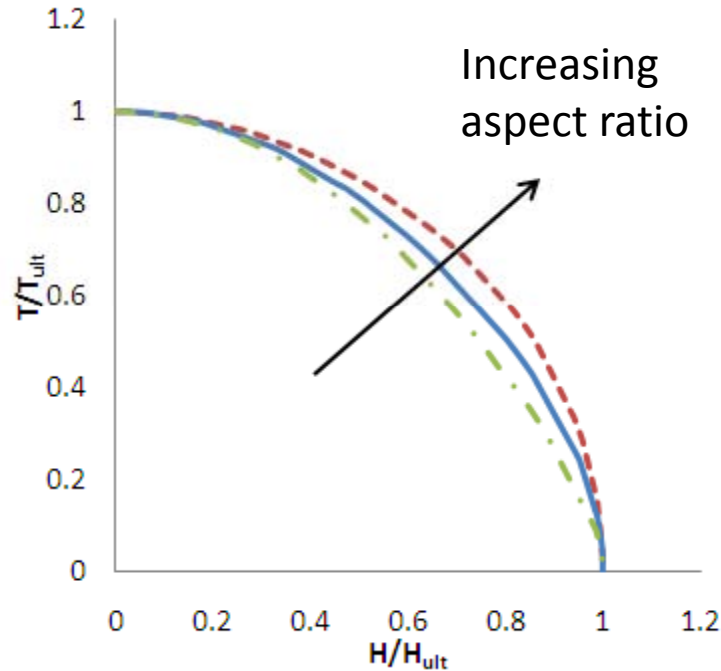


$B/L = 2$



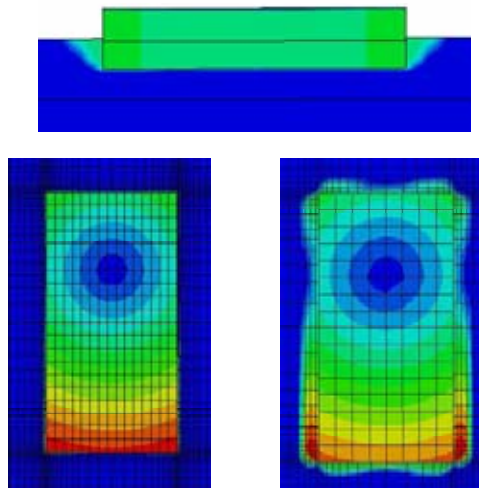
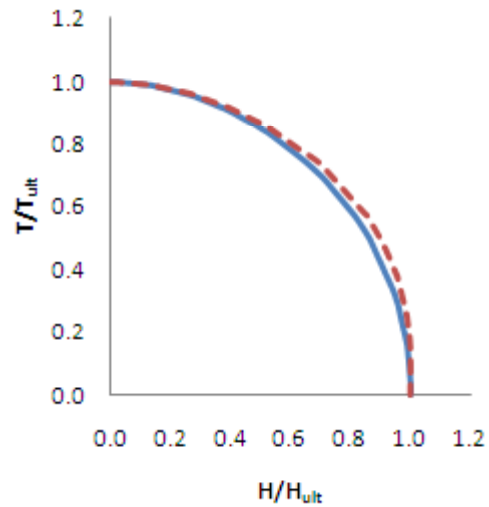
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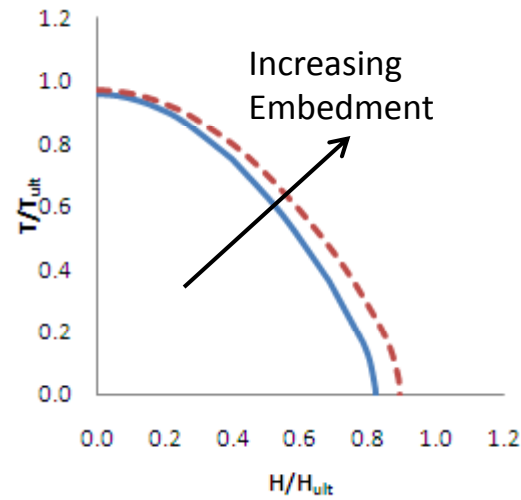


# Foundation embedment

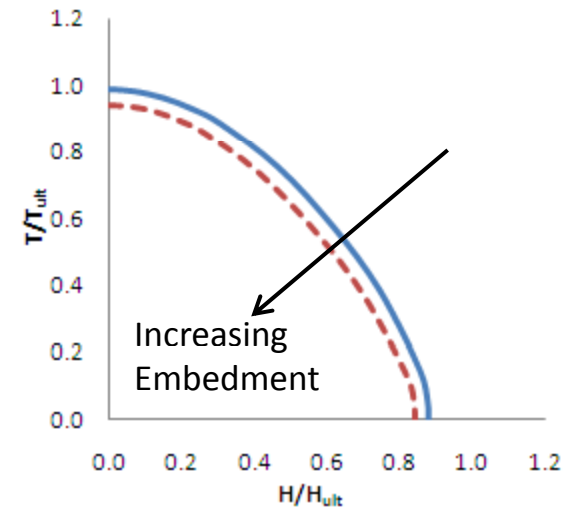
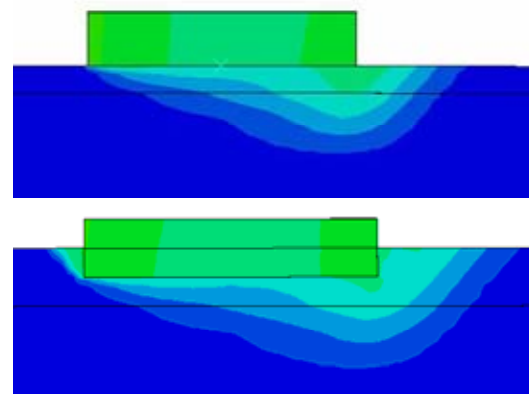
$V \leq 50\%$



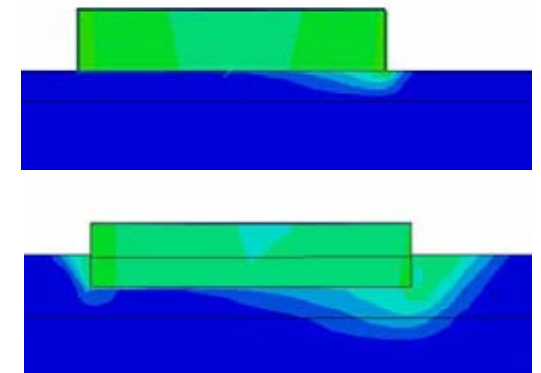
$V = 75\%$



Uniform soil

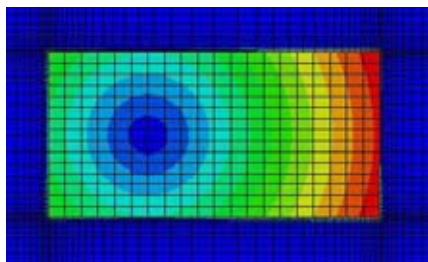
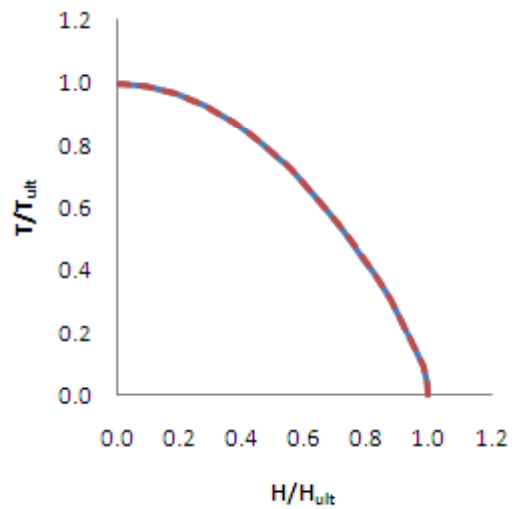


Non-uniform soil

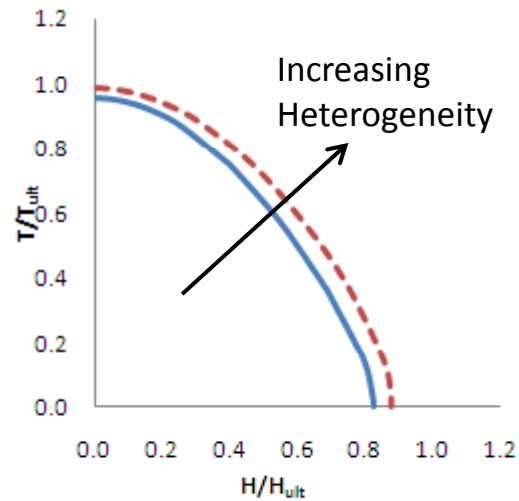


# Soil strength profile

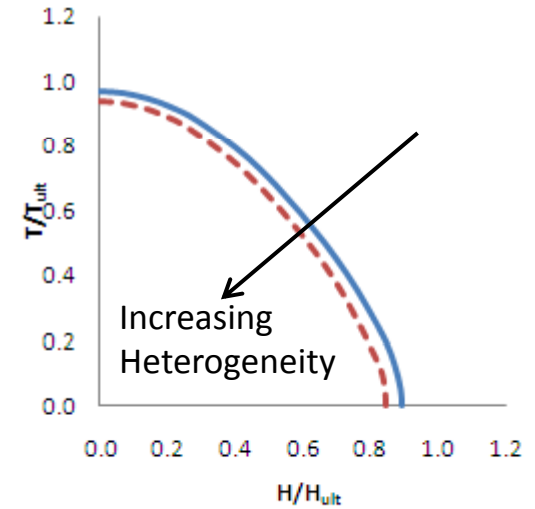
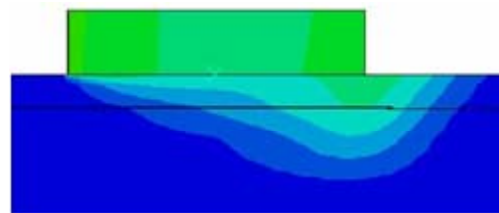
$V \leq 50\%$



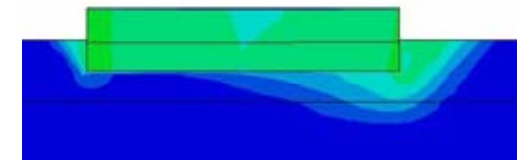
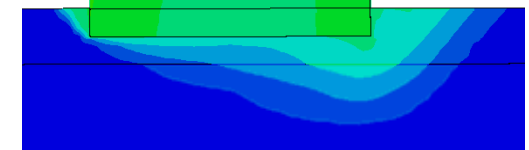
$V = 75\%$



Surface foundation



Embedded foundation





# Closed form solution

- Allows simple and accurate modelling of combined VHT loading

$$\left(\frac{h}{h^*}\right)^a + \left(\frac{t}{t^*}\right)^b - 1 = 0$$

- h = normalised horizontal loading
- t = normalised torsional loading
  
- h\* = available normalised horizontal capacity
- t\* = available normalised torsional capacity
  
- ‘a’ and ‘b’ are constants
  - $1.4 \leq a \leq 1.8$
  - $1.3 \leq b \leq 2.5$

# Conclusions

- Improvement to current design techniques
  - Classical Bearing capacity theory insufficient for offshore applications
  - VHT interaction curves provide better understanding
- When  $V \leq 50\%$ , embedment and heterogeneity can be ignored
- Closed form solutions
  - Allow for easy use in design



# Thankyou

- Thankyou to Professor Susan Gourvenec for her excellent supervision this year
- Thankyou to Ian Finnie from Advanced Geomechanics for his assistance with current design codes
- **Questions?**





# Closed form solution

- Surface foundation with  $B/L = 2$  on uniform soil body
- Take loads as:  $h = 0.7$ ,  $t = 0.6$ ,  $v < 0.5$
- For these conditions
  - $a = 1.7$ ,  $b = 2.1$
- When  $v \leq 0.5 \rightarrow h^* = 1$  and  $t^* = 1$

$$\left(\frac{0.7}{1}\right)^{1.7} + \left(\frac{0.6}{1}\right)^{2.1} - 1 = -0.11$$

- Result below zero, inside failure envelope

# Closed form solution

- Take vertical load as increased to  $v = 0.75$ , other factors the same  $\rightarrow a = 1.7, b = 2$

$$h^* = 1 \times \sqrt{1 - 1.6(0.75)^7 + 0.6(0.75)^{10}} = 0.906$$

$$t^* = 1.21 \times (0.75 - 1.6(0.75)^3 + 0.6(0.75)^{12})^{1/4} = 0.956$$

$$\left(\frac{0.7}{0.906}\right)^{1.7} + \left(\frac{0.6}{0.956}\right)^2 - 1 = 0.039$$

